

PREDICTION OF SEDIMENT TRANSPORT RATE IN IRRIGATION CANALS USING MODIFIED LAURSEN METHODOLOGY

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ABSTRACT

An improved method for predicting sediment transport in alluvial channels has been developed based upon modifications of the Laursen (1958) equation and based upon a wide range of field data. The applicability of this new method was tested for irrigation canals. The following canals were selected for the study: ACOP in Pakistan, American canals, Canal del Dique, CHOP in West Pakistan, India canals, and the Rio Grande Conveyance Canal, which have a range of flow depth from 0.67 m to 5.13 m and width from 3.19 m to 140.21 m. Comparisons between computed results from the modified formula and field data verify that the new method can be utilized to compute total bed-material load in canals.

INTRODUCTION

Many researchers have observed that more than one value of sediment transport can be obtained from the same values of Q , u , S_w , S_f , and τ , Simons and Senturk (1992) and Yang (1996). Gilbert (1914) first reported the phenomena that different values of transport can occur for the same hydraulic parameters. He concluded that there was no correlation at all between water discharge and sediment discharge. Accordingly, the validity of the assumption that total sediment discharge for a given particle size could be determined by the proposed parameters was questionable. Yang (1996) stated that stream power τu , identified as the independent variable, has a strong correlation with bed material transport. Lane (1955) and Bagnold (1966) first proposed this concept that stream power has a strong correlation with sediment discharge. Furthermore, Yang (1996) suggested that unit stream power uS has a stronger relationship to bed material transport than stream power. Other researchers suggested that the stream power-type relationships could be utilized in straight channels as well as channels that

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are in the process of changing their patterns from straight to meandering or braiding (Yang, 1977 and 1996; Vanoni, 1978; Yang and Molinas, 1982). It is important to note that in dimensionless form, use of the unit stream power uS/ω , further improved the correlation with bed material discharge (Yang and Kong, 1991). Simons and Senturk (1992) reported that channel gradient is a variable difficult to measure precisely. They suggested that the stream power parameter could be improved by inclusion of a velocity parameter to replace the slope of the channel.

Kodoatie (1999) developed an improved methodology for predicting sediment transport in alluvial channels based upon utilization of a wide range of recirculating flume and field data. A total of 4,532 data sets from 33 river systems as well as 919 data sets from flume studies were utilized to develop a new sediment transport relation to estimate total bed material transport based upon modification of the Laursen (1958) method. Kodoatie adopted the Laursen methodology for analysis because this methodology was expressed in terms that permitted identification and separation of the various parameters, which are generally considered to cover the important variables, related to bed-material transport. The Laursen equation is:

$$C_t = 0.01 \gamma \sum_i p_i \left(\frac{d_i}{d} \right)^{7/6} \left(\frac{\tau_{oi}'}{\tau_{ci}} - 1 \right) f \left(\frac{u_*}{\omega_i} \right) \quad (1)$$

The river data utilized by Kodoatie was compiled by the following individuals: (1) Brownlie (1981) and Posada (1995) for the Mississippi, the Amazon, the Orinoco, and the Apure River Systems; (2) Williams and Rosgen (1989) for the Black, the Chippewa, the Chulitna, the North Fork Toutle, the Susitna, the Toutle, the Wisconsin, and the Yampa River Systems; and (3) Long and Liang (1995) for the Yellow and the Yangtze River Systems. Brownlie (1981) and Willcock and Southard (1988) compiled the laboratory data utilized. The sets of field data were divided into two categories: Group 1 for developing the new proposed equations, and Group 2 for verification and validation. To develop these groups, the river data sets were divided into two groups in random order. Based on sediment size four river data sets were analyzed, including gravel-bed rivers, medium to very coarse sand-bed rivers, very fine to fine sand-bed rivers, and silt-bed rivers. The data were also divided according to the size of river in terms of width and depth. This division was based as follows:

1. small rivers with widths equal to or less than 10 m and depths equal to or less than 1 m,
2. intermediate rivers with widths greater than 10 m and widths equal to or less than 50 m and depths greater than 1 m and equal to or less than 3 m, and
3. large rivers with widths greater than 50 m and depths greater than 3 m.

MODIFICATIONS OF LAURSEN'S GRAPH

Madden (1985), utilizing Arkansas River data, modified Laursen's concepts. The modification by Madden permitted the analysis of bed material transport by size fractions. The Corps of Engineers' Waterways Experiment Station (1988) adopted this methodology for computing the transport in rivers with a mixture of sand and gravel forming the bed. Madden's modification to Laursen's methodology was based on three sets of special measurements made in the Arkansas River. The first two sets were gathered near Dardanelle in June and July 1957 and in April 1958. The third set was gathered near Morrilton in April 1958. Madden utilized the Missouri River data collected by Bondurant (1958) to validate the rating curves for the Arkansas River. The data sets resulted in best-fit curves that were parallel, but the two data sets did not overlap. The reader is reminded that the two sets of data were from two different rivers and even though they were sand-bed rivers, the methodologies produced by Bondurant and Madden were developed for specific rivers and are not generally applicable to other river systems. An adjustment factor related to the Froude number was utilized to modify the original Laursen equation by Madden (1985) as follows:

$$C_t = \sum_i p_i \left(\frac{d_i}{d} \right)^{7/6} \left(\frac{\tau_o'}{\tau_{ci}} - 1 \right) f \left(\frac{u_*}{\omega_i} \right) \left(\frac{0.1616}{F_r^{0.904}} \right) \quad (2)$$

In addition, Copeland and Thomas (1989) proposed a modification of Laursen's (1958) concept. This methodology was formulated to be utilized in both sand-bed rivers and, to a lesser degree, in larger gravel-bed rivers. The modified Laursen equation as presented by Copeland (1989) is:

$$C_t = 0.01 \gamma \sum_i p_i \left(\frac{d_i}{d} \right)^{7/6} \left(\frac{\tau_o'}{\tau_{ci}} - 1 \right) f \left(\frac{u_*'}{\omega_i} \right) \quad (3)$$

Copeland and Thomas' modification to the Laursen concepts as applied shows their results in Fig. 1.

Kodoatie (1999) modified the Laursen graph using field data as illustrated in Fig. 2. At an earlier date, Bondurant (1958) proposed a modification of Laursen's concepts. Bondurant developed a relationship between (u_*/ω) and $f(u_*/\omega)$ in the Laursen graph from Missouri River data at Omaha and at Kansas City. At the lower portion of Laursen graph, Bondurant's (1958) modification fits quite well. However, between the upper portion and the lower part of the relation, the values of u_*/ω for the Missouri River data deviate sharply from those of the Laursen graph.

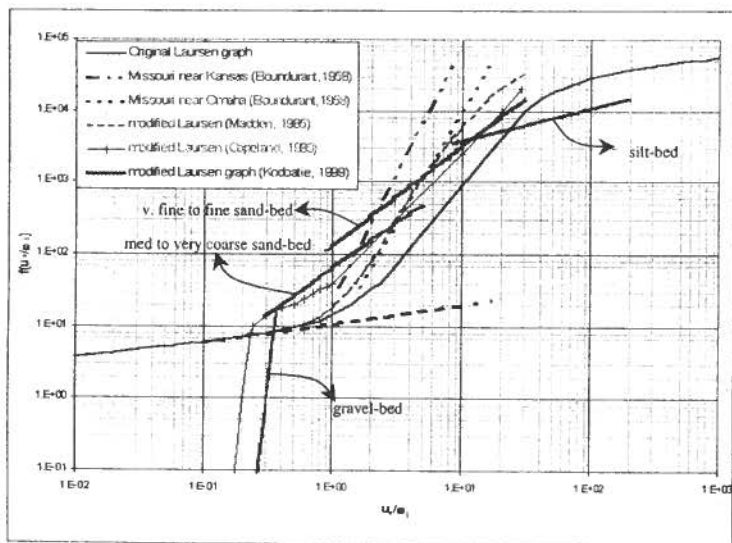


Fig. 1. Modifications of Laursen's (1958) Graph.

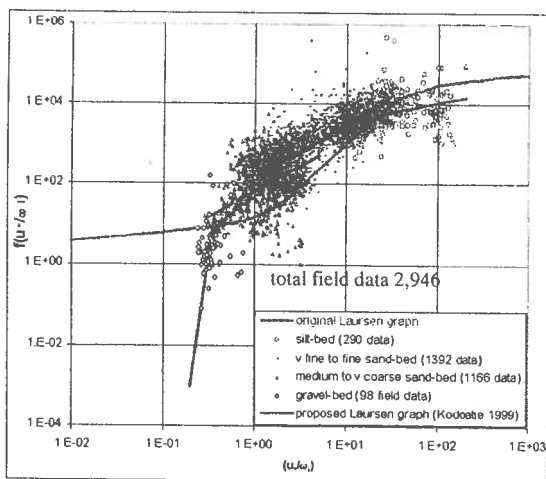


Fig. 2. Original Laursen and modified Laursen graphs by Kodoatie.

KODOATIE'S MODIFICATION OF THE LAURSEN METHODOLOGY

Kodoatie (1999) utilized a total number of data points (1,459) that pertained to transport of bed material. These data covered the range of particle size from gravels, medium to coarse sand, very fine to fine sand, and silt beds utilized only the dimensionless unit stream power uS/ω . Kodoatie verified that this parameter has a stronger correlation with bed material discharge than other stream power parameters. Figure 3a illustrates the scatter when stream power is plotted against measured concentration of suspended sediment. Figure 3b illustrates the scatter when unit stream power is correlated with measured concentration of suspended sediment. In both of these figures, there is significant scatter. In particular, the data points for gravel-bed rivers plot significantly above the data points for sand-bed and silt-bed rivers. Figure 3c verifies a much stronger relationship between measured concentration of suspended sediment and dimensionless stream power uS/ω . Additionally, the gravel-bed data commingles with the other data.

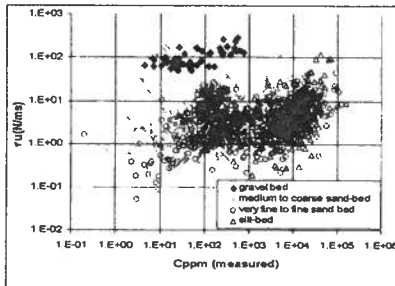
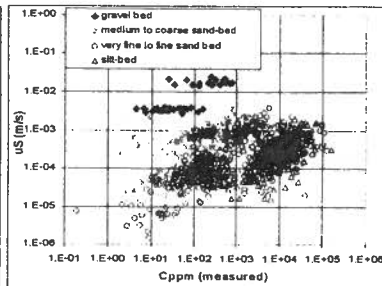
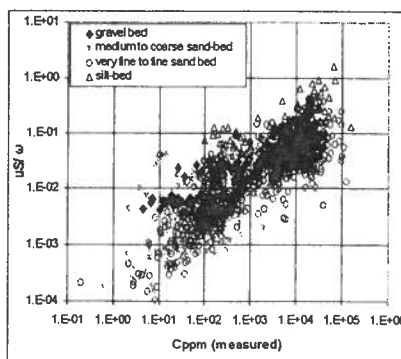
Fig. 3a. Stream Power τ Fig. 3b. Unit Stream Power uS Fig. 3c. Dimensionless Unit Stream Power uS/ω

Fig.3. Relations Among C_{ppm} (Measured), Stream Power, Unit Stream Power, and Dimensionless Unit Stream Power.

Summarizing the three figures, significant variation in these relationships still exists. For the bed material ranging in size from fine to coarse sand, it is difficult to ascertain differences in bed material transport in the relationship between dimensionless stream power and measured suspended concentration. For example, Fig. 3c indicates the same concentration of measured sediment for fine sand-bed rivers and coarse sand-bed rivers. However, the reduction in diameter from gravel-bed rivers to silt-bed rivers tends to increase measured suspended sediment concentration. The Pearson Correlation Coefficients for the analysis illustrated in Figs. 3a through 3c are presented in Tables 1a, 1b, and 1c, respectively.

Table 1. Pearson Correlation Coefficient between C_{ppm} Measured and uS/ω , uS and τu for Various Rivers.

A. Gravel-bed Rivers

	Measured C_{ppm}	uS/ω	$uS(m/s)$	$\tau u(N/ms)$
$C_{ppm}measured$	1			
uS/ω	0.71	1		
$uS(m/s)$	0.69	0.95	1	
$\tau u(N/ms)$	0.67	0.82	0.88	1

B. Medium to Coarse Sand-bed Rivers

	Measured C_{ppm}	uS/ω	$uS(m/s)$	$\tau u(N/ms)$
$C_{ppm}measured$	1			
uS/ω	0.58	1		
$uS(m/s)$	0.45	0.9	1	
$\tau u(N/ms)$	0.33	0.23	0.29	1

C. Very Fine to Fine Sand-bed Rivers

s	Measured C_{ppm}	uS/ω	$uS(m/s)$	$\tau u(N/ms)$
$C_{ppm}measured$	1			
uS/ω	0.74	1		
$uS(m/s)$	0.76	0.87	1	
$\tau u(N/ms)$	0.18	0.40	0.46	1

Table 1, continued

D. Silt-bed Rivers

	Measured C_{ppm}	uS/ω	$uS(m/s)$	$\tau u(N/ms)$
$C_{ppm \text{ measured}}$	1			
uS/ω	0.45	1		
$uS(m/s)$	0.62	0.79	1	
$\tau u(N/ms)$	0.30	0.74	0.78	1

In summary, the dimensionless unit stream power has the best correlation with measured suspended sediment C_{ppm} , see Fig. 3c. The strength of the correlation is exhibited in Fig. 4. This figure illustrates the range of dimensionless unit stream power and dimensionless flow depth compared to measured C_{ppm} . The dimensionless unit stream power utilizing regression analysis and nonlinear optimization techniques can add other variables to Equation 4 resulting in:

$$C_t = 0.01 \gamma \left(\frac{d_{50}}{d} \right)^{7/6} \left(\frac{\tau_o' - 1}{\tau_{c50}} - 1 \right) 10^{\left(\frac{u_*'}{\omega_{50}} \right)} \left(\frac{uS}{\omega} \right)^a \quad (4)$$

where a is a variable related to mean bed material diameter as shown in Table 2.

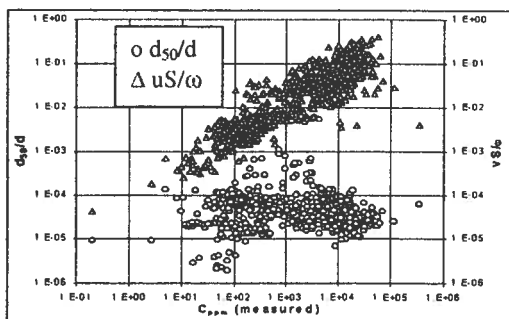


Fig. 4. Range of Dimensionless Unit Stream power and Dimensionless Flow Depth Compared to C_{ppm} Measured.

Equation 4 has been utilized to develop Fig. 5 illustrating the relationship u_*'/ω versus $\log(u_*'/\omega)$.

Table 2. Value of "a" in Equation (4) for Various Bed Materials.

Bed Material	"a"
Gravel	0
Medium to very coarse sand	-0.2
Very fine to fine sand	0.078
Silt	0.06

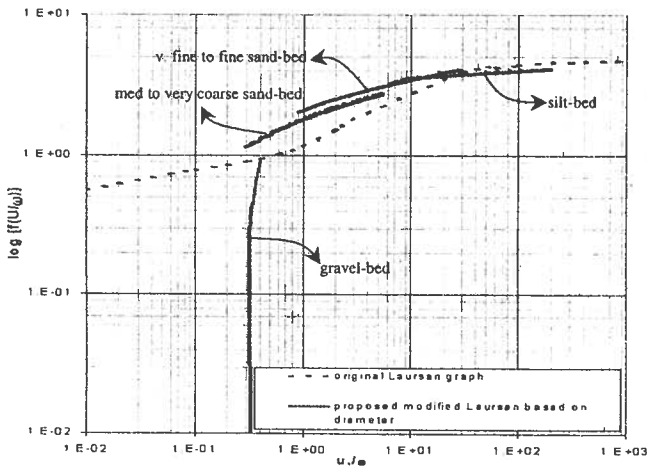


Fig. 5. Proposed Graph by Kodoatie Using Equation (4).

The modified Laursen equation by Copeland was applied by Raphael (1996) to 10 rivers including: the Toutle, North Fork Toutle, Susitna, Tanana, Oak Creek, Clearwater, Chippewa near Durand, Chippewa near Caryville and Granite Creek Rivers. Utilizing these data, which included 187 sets illustrating conditions in gravel-bed rivers, 147 computed data points were greater than measured values, and 39 data points were within the range of acceptable accuracy, as illustrated by the discrepancy ratio comparing computed measured values. The discrepancy ratio for these data had a range of 0.2 to 5. One set of data points was below an acceptable range of accuracy. Raphael concluded that the modified Laursen equation by Copeland significantly over-predicted gravel-bed material transport when applied by size fraction. The maximum over prediction was as much as 6,000 times higher than the measured value. The data falling within the range of acceptable accuracy all were for either sand- or very fine gravel-bed streams. The reduction of the critical dimensionless shear stress without taking into account the probability of grains to move and without including a hiding factor are probably the major reasons for over-prediction of bed material transport

for those streams with $D_{50} > 4.00$ mm. The modifications to the Laursen equation by Madden, Copeland and Kodoatie are presented in Table 3.

Table 3. Comparison of Modified Laursen Equations

1	Used the same equation; added adjustment factor related to Froude Number	1	Used grain hydraulic roughness instead of grain shear stress	1	Used same equation but added dimensionless stream power as the adjustment factor
2	Used modified graph	2	Used modified graph	2	Used modified graph; more specific in particle bed diameter from silt to gravel
3	Used size fraction	3	Used size fraction	3	Used uniform particle diameter (no fraction)
4	Used Arkansas River data	4	Used both river and flume data (not specified)	4	Used 33 river systems and 18 sources of flume data (more than 5,300 total sets)
5	Graph is higher than original for sand bed; not specified for gravel and silt	5	Graph is higher than original for sand bed; for silt not specified; graph for gravel is smaller than new proposed	5	Graph is higher than the original for sand bed (sand bed is more specific for very fine to fine sand and medium to very coarse sand); smaller for silt compared to original; graph for gravel is proposed

APPLICATION OF THE MODIFIED LAURSEN EQUATION TO TRANSPORT IN CANALS

In order to ascertain the effectiveness of the Kodoatie (1999) equation, it was applied to data sets obtained from irrigation canals. A total of 334 sets of canal data were selected and utilized to test this equation. The data were collected from an array of canals as follows:

- ACOP Canals (West Pakistan)
- CHOP Canals (West Pakistan)
- American Canals (Colorado, Wyoming, Nebraska)
- Canal del Dique
- India Canals
- Rio Grande Conveyance Channel

ACOP Canals

A total of 151 sets of canal data were collected Mahmood, et al. (1979) from five canals in West Pakistan. However, only 142 complete sets of data were utilized in this study.

American Canals

Simons (1957) collected 24 sets of data from canal systems. The canals utilized in Colorado include: Bijou 53 & 54, Fort. Morgan I, II, III, IV, and V; in Wyoming: Laramie I and IV, Garland I and II, Lucerne I and II, North Plate Ditch, the Farmers Canal, In Nebraska: Central Nebraska Public Power & Irrigation and Drainage Canal, Lateral A29.1, the Cozad Canal, the Dawson Canal and the Taylor Canal. Twelve of completed sets of canal data were utilized in this analysis.

Canal del Dique

A total of 61 sets of data were collected in this canal located in Columbia, South America by Nedeco (1973). Thirty two of these sets of data were utilized in this analysis. The data were collected at 10 stations along this canal.

CHOP Canals

A total of 33 data sets were collected from nine canals in West Pakistan. These data sets were collected by Chaudry, et al (1970) under the auspices of the Canal and Headworks Observation Program (CHOP) of the West Pakistan Water and Power Development Authority, 1962 – 1964, Lahore, West Pakistan.

India Canals

Chitale (1966) collected and reported 32 sets of canals in India.

Rio Grande Conveyance Channel

A total of nine sets of data were collected by Culbertson, et al (1972) at a station called Oak Creek.

The Range of Data Utilized From Canals

The range of data data utilized from canal studies include:

- discharge : 1.15 – 567.00 m³/s
- width : 3.19 – 140.21 m
- depth : 0.67 – 5.13 m
- D₅₀ : 0.020 (silt-bed) – 0.715 (sand-bed)
- Slope : 0.000003 – 0.00111

Analysis of Selected Canal Data

Kodoatie's modification of Laursen's equation was utilized to compare computed results with measured field data. Statistical methods were utilized including the mean discrepancy ratio $\overline{R}_D$ (Bechteller & Vetter, 1989; Nakato, 1990; Yang and Wan, 1991; Hydraul-Tech, Inc., 1998 and Wu, 1999) and the correlation coefficient C_c (Hydraul-Tech, Inc., 1998). The equations for each of these two statistical parameters follow.

$$\overline{R}_D = \sum \frac{R_i}{N}, \quad R_i = \frac{X_i}{Y_i} \quad (5)$$

$$C_c = \frac{\sum (X_i - \bar{X})(Y_i - \bar{Y})}{\sqrt{\sum (X_i - \bar{X})^2 \sum (Y_i - \bar{Y})^2}} \quad (6)$$

For perfect correlation, the above statistical parameters utilizing Eq. (5) and Eq. (6) require that $\overline{R}_D = 1$ and $C_c = 1$. The results of the analysis comparing measured C_{ppm} with computed C_{ppm} are shown in Figs. 6a, b, and c. The discrepancy ratio and Pearson correlation coefficients for three ranges of bed-material sizes dictated by the canal data analyzed are illustrated in Table 4. From Fig. 6 and Table 4, it is concluded that the original Laursen equation over-predicts for silt-bed canals, and under-predicts for sand-bed canals. This figure and table illustrate that the comparison of measured concentration with predicted concentration using the Kodoatie modification of Laursen's equation gives much improved results.

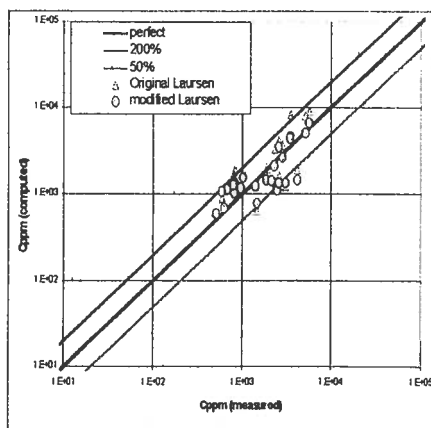


Fig. 6a. Silt-Bed Canals.

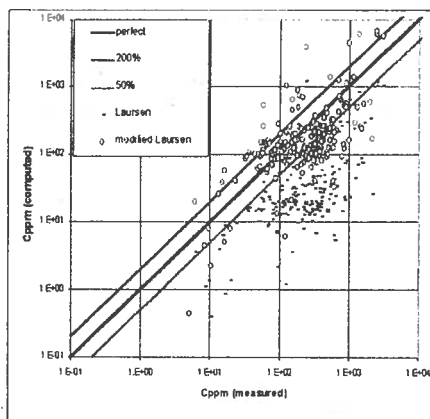


Fig. 6b. Very Fine to Fine Sand-Bed Canals.

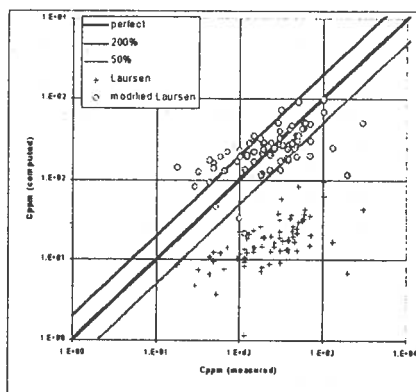


Fig. 6c. Medium to Very Coarse Sand-Bed Canals.

Fig. 6. C_{ppm} Computed and C_{ppm} Measured Data Utilizing Laursen And Modified Laursen.Table 4. Discrepancy Ratio $\overline{R_D}$ and Pearson Correlation Coefficient C_c Between C_{ppm} Measured and C_{ppm} Measured Using Original Laursen Equation and Modified Laursen Equation

Type of Canal Bed	C_c	C_c	$\overline{R_D}$	$\overline{R_D}$
	Original	Modified	Original	Modified
Silt	0.81	0.79	1.26	1.01
Very fine to fine to sand	0.65	0.64	0.06	1.15
Med. to very coarse sand	0.41	0.41	0.09	1.27

CONCLUSIONS

The utilization of such an extensive array of data encompassing a wide range of dimensions, gradients, and sizes of bed material has not been accomplished in past research. It is emphasized that:

- The geometry of a canal is selected based upon the type of bed and bank material, flow, variation in flow, and duration of range of flows as dictated by the irrigation/utilities' demand based upon season and climate.
- Unlined canals must be cognizant of the potential for erosion, aggradation and/or deposition.
- The proposed methodology for estimating transport of sediments in erodible canals is considerably improved over existing relationships. However, the transport of sediments in stable alluvial canals is only an improved estimate. In addition, it is recommended that the sorting of sediment through the canal should be done by size fractions to estimate changes in the median diameter of bed material with time and distance.
- Canals may have sediment-laden water and/or clean water at their headworks. If the supply of water and sediment are dependent upon the characteristics of the river, the design, and the operation of the diversion structure, etc., exclusion and ejection structures must be integrated into the design of the headworks.
- It is also concluded that during flood stages in the river the headworks should be closed if at all possible. Otherwise, considerable sediment may be permitted to enter the canal where it will deposit within a short distance of the headworks. This deposition could possibly reduce the capacity of the canal, and, as another alternative, steepen the gradient in a depositional area sufficiently to increase velocities to a level where considerable bank erosion may occur.

RECOMMENDATIONS

- There is a wide variation in predicted sediment transport in canals. Therefore, sediment exclusion and/or sediment ejection facilities may be required to refine the sediment supply to acceptable limits for a specific canal.
- Regime relations are recommended to estimate channel geometry for canals without linings constructed in erodible soil, Simons and Albertson (1963).

- The regime of the canal may change over time as a consequence of sorting of the natural soil that the canal is constructed within and the introduction of sediments at the headworks from the river into the canal.

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NOTATION

ρ	density of water
ω	fall velocity
γ	specific weight of water
τ_{ci}	critical shear stress for sediment size d_i
ω_i	fall velocity of the sediment size d_i
ω_i	fall velocity of particle diameter of the i^{th} size
τ_o	shear stress
τ_o'	bed shear stress due to grain size
τ_u	stream power
C_c	correlation coefficient and is defined by Equation (2.75)
C_i	concentration distribution in Equation (2.19)
C_t	total average sediment concentration in weight per unit volume
d	flow depth
D_{50}	mean diameter of sediment
d_i	geometric mean diameter of particle of the i^{th} size
F_r	Froude number
$f\left(\frac{u_*}{\omega_i}\right)$	functional relation of u_*/ω_i

$f\left(\frac{u'_*}{\omega_i}\right)$	functional relation of u'_*/ω_i , Plate 16 Copeland & Thomas (1989)
i	data set or point number
N	total number of data sets
p_{bi}	fraction of bed material of d_i
p_b	fraction of bed material
p_i	fraction of bed material for diameter particle size d_i
R_i	ratio of computed sedimentation and measured sedimentation
$\overline{R_D}$	mean discrepancy ratio
S	channel slope
u	mean velocity
u_*	shear velocity $\sqrt{\tau_o/\rho}$
u'_*	shear velocity due to grain roughness
uS	unit stream power
uS/ω	dimensionless unit stream power
X_i	computed sedimentation
Y_i	measured sedimentation
$\overline{X}$	average of computed concentration of sediment
$\overline{Y}$	average of measured concentration of sediment

DETAILS OF CANAL DATA

Total No	No of Each Canal	Canal Data	Water Discharge m ³ /s	Channel Width m	Flow Depth m	Flow Velocity m/s	Mean Bed Diameter d, mm	W/S Slope S _w m/m	Water Temp °C	Measured Sed Concn ppm	Bed Form	Source
1	2	3	4	5	6	7	8	9	10	11		
1	1	American Canal	12.59	11.73	1.83	0.59	0.096	0.000063	23	370	5	Simons (1957)
2	2	American Canal	1.56	3.49	0.80	0.56	0.173	0.000253	21	249	4	
3	3	American Canal	1.22	3.19	0.80	0.47	0.229	0.000294	21	406	3	
4	4	American Canal	29.18	22.19	2.53	0.52	0.253	0.000058	22	115	3	
5	5	American Canal	29.40	14.81	2.59	0.77	0.311	0.000120	22	185	3	
6	6	American Canal	4.14	9.33	1.07	0.41	0.318	0.000135	25	254	3	
7	7	American Canal	3.20	3.96	1.32	0.61	0.349	0.000110	26	44	5	
8	8	American Canal	10.47	6.92	1.60	0.95	0.360	0.000114	26	131	5	
9	9	American Canal	6.42	12.56	1.01	0.50	0.446	0.000218	28	100	3	
10	10	American Canal	4.83	10.73	0.89	0.51	0.465	0.000237	26	52	3	
11	11	American Canal	5.01	7.62	0.89	0.74	0.580	0.000330	26	448	4	
12	12	American Canal	5.62	7.59	1.01	0.73	0.715	0.000302	23	123	4	
13	1	India Canal	156.05	56.27	3.39	0.82	0.020	0.000060	20	2,601	0	Chitale (1966)
14	2	India Canal	59.16	25.49	2.44	0.95	0.021	0.000084	20	5,759	0	
15	3	India Canal	153.25	56.02	3.37	0.81	0.024	0.000060	20	2,887	0	
16	4	India Canal	60.72	25.56	2.49	0.95	0.025	0.000084	20	5,182	0	
17	5	India Canal	157.41	56.47	3.35	0.83	0.030	0.000070	20	2,316	0	
18	6	India Canal	68.82	25.77	2.55	1.05	0.031	0.000110	20	3,557	0	
19	7	India Canal	27.67	17.98	2.52	0.61	0.033	0.000070	20	831	0	
20	8	India Canal	68.92	25.68	2.55	1.05	0.033	0.000110	20	3,508	0	
21	9	India Canal	14.11	13.49	1.85	0.57	0.036	0.000080	20	1,894	0	
22	10	India Canal	14.03	14.66	1.72	0.56	0.037	0.000080	20	4,230	0	
23	11	India Canal	27.50	17.89	2.51	0.61	0.039	0.000070	20	822	0	
24	12	India Canal	158.37	56.61	3.35	0.83	0.039	0.000070	20	2,175	0	
25	13	India Canal	2.02	5.34	0.94	0.40	0.042	0.000115	20	1,417	0	
26	14	India Canal	14.81	13.55	1.86	0.59	0.043	0.000080	20	2,467	0	
27	15	India Canal	33.74	20.58	2.38	0.69	0.043	0.000080	20	797	0	
28	16	India Canal	19.45	16.03	2.38	0.51	0.044	0.000088	20	624	0	
29	17	India Canal	3.00	5.78	1.10	0.47	0.046	0.000115	20	3,132	0	
30	18	India Canal	24.59	18.07	2.24	0.61	0.046	0.000120	20	2,517	0	
31	19	India Canal	1.15	4.35	0.67	0.39	0.048	0.000145	20	1,031	0	
32	20	India Canal	19.22	15.95	2.37	0.51	0.048	0.000088	20	512	0	
33	21	India Canal	13.41	10.58	1.97	0.65	0.050	0.000100	20	981	0	
34	22	India Canal	27.89	18.17	2.17	0.71	0.050	0.000112	20	2,601	0	
35	23	India Canal	13.19	10.70	1.94	0.64	0.051	0.000100	20	671	0	
36	24	India Canal	15.87	17.34	1.57	0.58	0.056	0.000120	20	596	0	
37	25	India Canal	242.19	79.10	3.56	0.86	0.057	0.000064	20	1,490	0	
38	26	India Canal	1.29	4.31	0.79	0.38	0.064	0.000165	20	760	0	
39	27	India Canal	30.86	20.57	2.37	0.63	0.064	0.000080	20	918	0	
40	28	India Canal	132.80	51.90	3.29	0.78	0.064	0.000065	20	1,976	0	
41	29	India Canal	131.39	51.51	3.29	0.77	0.066	0.000065	20	1,593	0	
42	30	India Canal	15.84	17.31	1.57	0.58	0.070	0.000120	20	726	0	
43	31	India Canal	166.36	66.54	3.41	0.73	0.080	0.000057	20	1,425	0	
44	32	India Canal	163.72	66.55	3.39	0.73	0.082	0.000057	20	1,519	0	
45	1	Pakistan Canal	158.09	118.87	2.23	0.60	0.083	0.000070	28	369	5	Mahmood et al (1979)
46	2	Pakistan Canal	94.27	88.39	1.46	0.73	0.084	0.000137	28	190	5	
47	3	Pakistan Canal	29.59	35.66	1.68	0.49	0.085	0.000085	31	103	0	
48	4	Pakistan Canal	76.94	69.49	1.83	0.61	0.108	0.000132	27	125	0	
49	5	Pakistan Canal	52.13	35.66	2.29	0.64	0.110	0.000075	32	156	0	
50	6	Pakistan Canal	54.14	35.36	2.26	0.68	0.112	0.000067	21	61	0	
51	7	Pakistan Canal	55.16	35.97	2.23	0.69	0.112	0.000085	22	289	0	
52	8	Pakistan Canal	55.64	35.66	2.35	0.66	0.113	0.000077	28	511	0	
53	9	Pakistan Canal	528.68	123.44	3.72	1.15	0.113	0.000055	23	95	0	
54	10	Pakistan Canal	51.42	35.66	2.32	0.62	0.114	0.000070	23	76	5	
55	11	Pakistan Canal	48.62	35.66	2.32	0.59	0.116	0.000072	23	32	0	
56	12	Pakistan Canal	151.24	90.22	1.89	0.89	0.116	0.000152	29	188	0	
57	13	Pakistan Canal	52.41	35.66	2.53	0.58	0.117	0.000076	28	128	3	
58	14	Pakistan Canal	138.04	70.41	2.41	0.81	0.118	0.000149	26	563	0	

59	15	Pakistan Canal	52.27	35.36	2.26	0.66	0.121	0.000073	23	58	0
60	16	Pakistan Canal	58.33	35.97	2.47	0.66	0.122	0.000087	29	166	0
61	17	Pakistan Canal	51.88	35.36	2.19	0.67	0.123	0.000086	17	422	0
62	18	Pakistan Canal	52.47	35.36	2.29	0.65	0.123	0.000088	24	869	5
63	19	Pakistan Canal	54.74	35.66	2.19	0.70	0.124	0.000089	16	560	0
64	20	Pakistan Canal	130.46	70.41	2.35	0.79	0.125	0.000129	25	297	0
65	21	Pakistan Canal	140.14	70.10	2.35	0.85	0.126	0.000134	28	564	0
66	22	Pakistan Canal	61.73	35.66	2.53	0.68	0.127	0.000074	29	79	0
67	23	Pakistan Canal	27.50	86.26	0.91	0.35	0.128	0.000142	18	15	0
68	24	Pakistan Canal	50.60	35.66	2.23	0.64	0.128	0.000074	24	54	5
69	25	Pakistan Canal	54.37	35.36	2.19	0.70	0.129	0.000085	18	367	0
70	26	Pakistan Canal	96.59	86.56	1.62	0.69	0.131	0.000139	26	331	0
71	27	Pakistan Canal	52.92	35.36	2.16	0.69	0.132	0.000085	18	153	0
72	28	Pakistan Canal	137.82	69.19	2.38	0.84	0.132	0.000133	28	607	0
73	29	Pakistan Canal	136.32	127.41	1.77	0.61	0.133	0.000086	28	34	0
74	30	Pakistan Canal	51.90	35.66	2.19	0.66	0.136	0.000086	18	386	0
75	31	Pakistan Canal	56.29	35.97	2.29	0.68	0.138	0.000076	24	184	0
76	32	Pakistan Canal	52.13	49.07	1.43	0.74	0.140	0.000148	31	445	0
77	33	Pakistan Canal	70.40	46.63	2.16	0.70	0.142	0.000101	25	346	0
78	34	Pakistan Canal	74.11	46.63	2.23	0.71	0.142	0.000116	32	233	0
79	35	Pakistan Canal	79.17	46.94	2.44	0.69	0.142	0.000095	21	383	0
80	36	Pakistan Canal	70.42	46.63	2.19	0.69	0.143	0.000115	30	82	0
81	37	Pakistan Canal	56.80	46.33	1.92	0.64	0.144	0.000109	30	225	0
82	38	Pakistan Canal	70.40	46.63	2.32	0.65	0.144	0.000116	29	323	3
83	39	Pakistan Canal	153.82	71.63	2.35	0.91	0.144	0.000166	26	584	3
84	40	Pakistan Canal	73.60	46.63	2.26	0.70	0.145	0.000111	31	335	0
85	41	Pakistan Canal	70.71	46.63	2.16	0.70	0.146	0.000099	25	366	3
86	42	Pakistan Canal	74.30	94.79	1.49	0.52	0.146	0.000142	26	77	0
87	43	Pakistan Canal	49.64	46.63	1.68	0.63	0.147	0.000110	30	36	0
88	44	Pakistan Canal	67.71	46.63	2.16	0.67	0.147	0.000110	36	372	0
89	45	Pakistan Canal	77.28	46.63	2.26	0.73	0.147	0.000109	31	577	0
90	46	Pakistan Canal	97.44	70.41	2.10	0.66	0.147	0.000134	26	164	0
91	47	Pakistan Canal	71.92	46.63	2.32	0.67	0.148	0.000112	28	796	0
92	48	Pakistan Canal	75.86	46.63	2.32	0.70	0.148	0.000115	31	351	0
93	49	Pakistan Canal	90.53	88.39	1.52	0.67	0.148	0.000137	28	183	0
94	50	Pakistan Canal	68.13	46.03	2.07	0.71	0.149	0.000127	21	529	3
95	51	Pakistan Canal	77.11	46.33	2.26	0.74	0.149	0.000108	31	289	0
96	52	Pakistan Canal	78.55	46.63	2.32	0.73	0.149	0.000107	32	385	3
97	53	Pakistan Canal	139.74	71.93	2.23	0.87	0.149	0.000107	25	391	0
98	54	Pakistan Canal	72.66	46.63	2.32	0.67	0.150	0.000116	25	142	0
99	55	Pakistan Canal	65.92	46.63	2.16	0.65	0.151	0.000107	25	290	3
100	56	Pakistan Canal	29.48	93.27	0.76	0.41	0.152	0.000088	17	16	0
101	57	Pakistan Canal	67.11	46.94	2.13	0.67	0.152	0.000112	30	54	0
102	58	Pakistan Canal	68.84	47.85	2.19	0.66	0.152	0.000148	15	410	3
103	59	Pakistan Canal	74.84	46.63	2.29	0.70	0.152	0.000107	28	304	0
104	60	Pakistan Canal	70.03	47.24	2.16	0.68	0.153	0.000116	30	48	3
105	61	Pakistan Canal	28.77	85.65	0.91	0.37	0.154	0.000124	18	13	0
106	62	Pakistan Canal	92.79	100.89	1.40	0.66	0.154	0.000106	27	106	4
107	63	Pakistan Canal	297.16	128.32	2.56	0.90	0.154	0.000097	25	2,083	0
108	64	Pakistan Canal	68.78	47.24	2.23	0.65	0.155	0.000147	16	845	3
109	65	Pakistan Canal	74.22	46.63	2.29	0.70	0.155	0.000114	28	333	0
110	66	Pakistan Canal	63.77	47.55	2.16	0.62	0.156	0.000150	16	110	0
111	67	Pakistan Canal	72.66	46.63	2.35	0.66	0.156	0.000112	29	310	0
112	68	Pakistan Canal	391.06	92.05	3.66	1.16	0.157	0.000150	30	342	0
113	69	Pakistan Canal	67.85	46.63	2.13	0.68	0.159	0.000124	25	146	0
114	70	Pakistan Canal	75.32	46.33	2.29	0.71	0.161	0.000104	28	240	0
115	71	Pakistan Canal	156.48	72.24	2.41	0.90	0.161	0.000149	27	228	4
116	72	Pakistan Canal	169.08	72.24	2.47	0.95	0.162	0.000121	28	169	0
117	73	Pakistan Canal	69.18	46.94	2.16	0.68	0.164	0.000102	25	262	0
118	74	Pakistan Canal	85.29	88.39	1.46	0.66	0.164	0.000129	26	132	0
119	75	Pakistan Canal	169.67	70.71	1.89	1.27	0.164	0.000134	30	872	0
120	76	Pakistan Canal	78.92	99.67	1.34	0.59	0.167	0.000104	26	88	0
121	77	Pakistan Canal	84.02	49.38	2.16	0.79	0.167	0.000146	31	56	3
122	78	Pakistan Canal	98.94	86.56	1.65	0.69	0.167	0.000127	26	232	0
123	79	Pakistan Canal	85.21	47.85	2.10	0.85	0.168	0.000132	21	322	0
124	80	Pakistan Canal	47.83	46.33	1.80	0.57	0.169	0.000108	32	215	0
125	81	Pakistan Canal	321.51	119.79	3.41	0.79	0.169	0.000088	14	138	0
126	82	Pakistan Canal	86.34	49.38	2.16	0.81	0.170	0.000147	29	1,007	0
127	83	Pakistan Canal	412.04	111.86	3.63	1.02	0.170	0.000119	25	216	0
128	84	Pakistan Canal	90.42	49.38	2.13	0.86	0.173	0.000153	21	517	0
129	85	Pakistan Canal	183.83	91.14	2.19	0.92	0.173	0.000138	26	373	0
130	86	Pakistan Canal	153.31	70.71	2.10	1.03	0.174	0.000135	30	419	4

131	87	Pakistan Canal	87.50	49.38	2.19	0.81	0.176	0.000144	32	328	0
132	88	Pakistan Canal	291.27	128.63	2.59	0.87	0.176	0.000100	23	115	0
133	89	Pakistan Canal	44.00	48.77	1.34	0.67	0.177	0.000145	31	94	0
134	90	Pakistan Canal	80.19	49.07	2.04	0.80	0.178	0.000148	25	69	3
135	91	Pakistan Canal	83.31	49.07	2.07	0.82	0.179	0.000145	32	122	0
136	92	Pakistan Canal	110.72	71.02	2.16	0.72	0.179	0.000137	26	481	0
137	93	Pakistan Canal	166.87	90.53	1.89	0.98	0.179	0.000113	28	319	0
138	94	Pakistan Canal	81.52	49.07	2.13	0.78	0.182	0.000152	20	399	0
139	95	Pakistan Canal	380.58	101.50	2.90	1.29	0.182	0.000105	30	57	0
140	96	Pakistan Canal	89.48	49.68	2.16	0.83	0.185	0.000154	16	104	0
141	97	Pakistan Canal	86.25	49.68	2.23	0.78	0.186	0.000154	17	167	3
142	98	Pakistan Canal	297.41	129.24	2.62	0.88	0.187	0.000098	19	229	0
143	99	Pakistan Canal	79.60	50.60	2.13	0.74	0.191	0.000154	16	279	0
144	100	Pakistan Canal	99.48	86.56	1.65	0.70	0.192	0.000129	26	289	0
145	101	Pakistan Canal	75.69	49.07	2.16	0.71	0.193	0.000148	34	98	0
146	102	Pakistan Canal	68.92	124.97	1.46	0.38	0.195	0.000045	19	5	0
147	103	Pakistan Canal	224.44	120.70	2.50	0.74	0.195	0.000082	31	65	0
148	104	Pakistan Canal	404.62	114.30	3.57	0.99	0.197	0.000119	14	614	0
149	105	Pakistan Canal	146.62	126.49	1.71	0.68	0.198	0.000087	30	48	0
150	106	Pakistan Canal	486.82	123.44	4.27	0.92	0.199	0.000103	29	205	3
151	107	Pakistan Canal	357.76	111.25	3.54	0.91	0.201	0.000125	15	162	0
152	108	Pakistan Canal	417.42	112.47	3.66	1.01	0.202	0.000117	25	114	0
153	109	Pakistan Canal	451.40	121.92	4.24	0.87	0.202	0.000098	27	67	0
154	110	Pakistan Canal	349.97	112.17	3.47	0.90	0.205	0.000112	13	106	0
155	111	Pakistan Canal	233.10	140.21	2.07	0.80	0.206	0.000098	12	268	5
156	112	Pakistan Canal	86.54	49.68	2.16	0.80	0.207	0.000154	17	71	3
157	113	Pakistan Canal	441.29	120.40	4.08	0.90	0.208	0.000093	23	181	0
158	114	Pakistan Canal	428.15	114.00	3.69	1.02	0.210	0.000122	13	54	0
159	115	Pakistan Canal	71.30	46.63	2.10	0.73	0.211	0.000104	24	79	3
160	116	Pakistan Canal	414.47	111.56	3.69	1.01	0.214	0.000121	23	86	0
161	117	Pakistan Canal	412.72	118.26	3.60	0.97	0.220	0.000121	21	57	0
162	118	Pakistan Canal	395.13	113.08	3.81	0.92	0.222	0.000061	27	89	0
163	119	Pakistan Canal	179.56	124.36	2.04	0.71	0.223	0.000112	21	52	0
164	120	Pakistan Canal	222.09	128.02	2.26	0.77	0.226	0.000104	12	97	0
165	121	Pakistan Canal	349.46	111.56	3.41	0.92	0.230	0.000121	15	44	3
166	122	Pakistan Canal	412.29	118.26	3.63	0.96	0.233	0.000120	23	106	0
167	123	Pakistan Canal	371.80	113.39	3.51	0.94	0.234	0.000120	15	493	0
168	124	Pakistan Canal	110.12	125.58	1.86	0.47	0.241	0.000086	17	19	0
169	125	Pakistan Canal	393.21	111.86	3.57	0.99	0.242	0.000121	24	108	0
170	126	Pakistan Canal	342.95	110.95	3.29	0.94	0.250	0.000119	16	33	0
171	127	Pakistan Canal	346.71	110.64	3.32	0.94	0.252	0.000123	14	71	0
172	128	Pakistan Canal	423.25	118.26	3.63	0.99	0.258	0.000121	24	59	4
173	129	Pakistan Canal	363.33	128.02	2.77	1.02	0.260	0.000093	26	265	0
174	130	Pakistan Canal	375.65	125.88	3.81	0.78	0.268	0.000116	-1	17	0
175	131	Pakistan Canal	362.91	120.09	3.57	0.85	0.272	0.000112	14	116	0
176	132	Pakistan Canal	207.62	126.19	2.16	0.76	0.273	0.000099	14	49	0
177	133	Pakistan Canal	363.64	110.95	3.44	0.95	0.275	0.000120	16	103	0
178	134	Pakistan Canal	392.70	119.79	3.57	0.92	0.275	0.000112	14	94	0
179	135	Pakistan Canal	387.69	122.23	3.72	0.85	0.279	0.000107	15	32	0
180	136	Pakistan Canal	394.77	117.65	3.54	0.95	0.279	0.000113	16	44	0
181	137	Pakistan Canal	337.28	116.43	3.20	0.91	0.289	0.000112	14	39	0
182	138	Pakistan Canal	279.80	135.94	2.32	0.89	0.293	0.000112	28	123	4
183	139	Pakistan Canal	355.72	116.74	3.26	0.93	0.299	0.000109	14	42	3
184	140	Pakistan Canal	267.65	117.35	2.80	0.81	0.313	0.000112	18	65	3
185	141	Pakistan Canal	388.05	121.92	3.66	0.87	0.331	0.000108	-1	18	0
186	142	Pakistan Canal	481.92	123.44	4.30	0.91	0.364	0.000079	18	29	0
187	1	Rio Grande Conveyance Canal	36.51	27.43	0.89	1.50	0.180	0.000520	17	985	5
188	2	Rio Grande Conveyance Canal	35.38	22.56	1.10	1.43	0.180	0.000660	4	2,486	5
189	3	Rio Grande Conveyance Canal	35.38	22.56	1.11	1.42	0.180	0.000590	3	3,049	5
190	4	Rio Grande Conveyance Canal	33.68	22.56	0.89	1.67	0.200	0.000730	17	1,348	4
191	5	Rio Grande Conveyance Canal	25.19	22.86	1.50	0.74	0.210	0.000650	15	906	3
192	6	Rio Grande Conveyance Canal	36.22	21.34	1.02	1.66	0.210	0.001110	18	2,475	4
193	7	Rio Grande Conveyance Canal	25.19	20.12	1.25	1.00	0.230	0.000650	15	906	3
194	8	Rio Grande Conveyance Canal	25.75	21.34	1.30	0.93	0.270	0.000650	15	1,025	3
195	9	Rio Grande Conveyance Canal	25.75	22.86	1.37	0.82	0.280	0.000650	15	1,025	3

Culbertson
et al.
(1972)

196	1	Canal del Dique	421.00	135.00	3.82	0.82	0.210	0.000057	30	626	0	Nedeco (1973)
197	2	Canal del Dique	567.00	140.00	4.60	0.88	0.210	0.000071	30	590.8	0	
198	3	Canal del Dique	478.00	120.00	3.55	1.12	0.210	0.000062	30	266.6	0	
199	4	Canal del Dique	83.00	108.00	2.47	0.31	0.210	0.000026	30	11.2	0	
200	5	Canal del Dique	158.00	114.00	2.65	0.52	0.210	0.000036	30	120.7	0	
201	6	Canal del Dique	440.00	85.00	4.47	1.16	0.150	0.000059	30	232.7	0	
202	7	Canal del Dique	544.00	84.00	5.13	1.26	0.150	0.000089	30	156.5	0	
203	8	Canal del Dique	120.00	76.00	3.54	0.45	0.150	0.000020	30	10.2	0	
204	9	Canal del Dique	111.00	41.00	4.12	0.66	0.100	0.000091	30	163.5	0	
205	10	Canal del Dique	142.00	36.00	3.92	1.01	0.100	0.000068	30	393.2	0	
206	11	Canal del Dique	55.00	39.00	3.08	0.46	0.100	0.000089	30	23	0	
207	12	Canal del Dique	57.00	31.00	3.42	0.54	0.100	0.000020	30	5.9	0	
208	13	Canal del Dique	322.00	69.00	3.78	1.23	0.120	0.000091	30	328.8	0	
209	14	Canal del Dique	280.00	85.00	3.15	1.05	0.120	0.000077	30	192.8	0	
210	15	Canal del Dique	370.00	88.00	3.91	1.08	0.120	0.000089	30	140.9	0	
211	16	Canal del Dique	68.00	74.00	2.16	0.43	0.120	0.000020	30	9.3	0	
212	17	Canal del Dique	89.00	32.00	2.68	1.04	0.185	0.000170	30	205.4	0	
213	18	Canal del Dique	126.00	34.00	2.62	1.41	0.185	0.000150	30	151.6	0	
214	19	Canal del Dique	80.00	30.00	2.57	1.04	0.185	0.000170	30	261.3	0	
215	20	Canal del Dique	29.00	27.00	2.21	0.49	0.185	0.000020	30	8.3	0	
216	21	Canal del Dique	169.00	95.00	2.21	0.80	0.120	0.000051	30	163	0	
217	22	Canal del Dique	228.00	90.00	2.78	0.91	0.120	0.000041	30	168.7	0	
218	23	Canal del Dique	215.00	100.00	2.93	0.73	0.120	0.000047	30	222.3	0	
219	24	Canal del Dique	51.00	93.00	1.80	0.30	0.120	0.000010	30	2.9	0	
220	25	Canal del Dique	225.00	86.00	2.93	0.89	0.125	0.000041	30	215.8	0	
221	26	Canal del Dique	38.00	77.00	2.49	0.20	0.125	0.000003	30	16.7	0	
222	27	Canal del Dique	150.00	45.00	2.62	1.27	0.125	0.000142	30	215.9	0	
223	28	Canal del Dique	128.00	35.00	2.96	1.24	0.125	0.000129	30	58.3	0	
224	29	Canal del Dique	153.00	41.00	2.63	1.42	0.125	0.000177	30	124.1	0	
225	30	Canal del Dique	82.00	75.00	2.08	0.53	0.125	0.000024	30	148.4	0	
226	31	Canal del Dique	95.00	75.00	1.87	0.68	0.125	0.000046	30	118.9	0	
227	32	Canal del Dique	81.00	78.00	1.82	0.57	0.125	0.000035	30	89.2	0	
228	1	West Pakistan (CHOP) Canal	172.44	112.78	1.31	1.17	0.090	0.000194	16	232	0	Chaudry et al. (1970)
229	2	West Pakistan (CHOP) Canal	120.91	55.47	2.44	0.89	0.100	0.000200	20	181	0	
230	3	West Pakistan (CHOP) Canal	109.58	57.91	2.68	0.71	0.110	0.000080	24	146	0	
231	4	West Pakistan (CHOP) Canal	233.61	118.26	2.47	0.80	0.110	0.000214	15	148	0	
232	5	West Pakistan (CHOP) Canal	146.68	67.67	2.68	0.81	0.120	0.000232	24	473	0	
233	6	West Pakistan (CHOP) Canal	359.61	111.25	2.10	1.54	0.120	0.000116	23	531	0	
234	7	West Pakistan (CHOP) Canal	362.44	99.06	3.08	1.19	0.120	0.000118	29	464	0	
235	8	West Pakistan (CHOP) Canal	351.12	112.17	2.13	1.47	0.130	0.000124	18	706	0	
236	9	West Pakistan (CHOP) Canal	424.74	111.86	2.38	1.60	0.140	0.000155	19	1,153	0	
237	10	West Pakistan (CHOP) Canal	166.75	67.67	2.56	0.96	0.190	0.000051	27	116	0	
238	11	West Pakistan (CHOP) Canal	27.52	23.77	1.68	0.69	0.200	0.000086	29	286	0	
239	12	West Pakistan (CHOP) Canal	112.41	57.30	2.32	0.85	0.200	0.000134	24	595	0	
240	13	West Pakistan (CHOP) Canal	322.80	120.40	2.68	1.00	0.200	0.000196	22	526	0	
241	14	West Pakistan (CHOP) Canal	362.44	118.26	2.99	1.03	0.200	0.000161	18	663	0	
242	15	West Pakistan (CHOP) Canal	393.59	99.67	3.38	1.17	0.200	0.000181	17	299	0	
243	16	West Pakistan (CHOP) Canal	413.41	110.64	2.44	1.53	0.200	0.000115	18	1,297	0	
244	17	West Pakistan (CHOP) Canal	209.26	71.63	3.32	0.88	0.210	0.000127	22	484	0	
245	18	West Pakistan (CHOP) Canal	328.47	97.54	3.32	1.01	0.210	0.000188	19	432	0	
246	19	West Pakistan (CHOP) Canal	334.13	110.34	2.47	1.23	0.210	0.000159	20	428	0	
247	20	West Pakistan (CHOP) Canal	342.62	116.74	3.11	0.94	0.210	0.000254	22	620	0	
248	21	West Pakistan (CHOP) Canal	376.60	115.82	2.35	1.39	0.210	0.000141	21	702	0	

249	22	West Pakistan (CHOP) Canal	427.57	121.62	3.17	1.11	0.210	0.000202	16	1,217	0
250	23	West Pakistan (CHOP) Canal	146.11	55.78	2.62	1.00	0.290	0.000182	27	244	0
251	24	West Pakistan (CHOP) Canal	153.47	58.52	2.71	0.97	0.290	0.000165	24	261	0
252	25	West Pakistan (CHOP) Canal	255.41	112.17	2.56	0.89	0.290	0.000207	12	305	0
253	26	West Pakistan (CHOP) Canal	114.96	57.91	2.35	0.85	0.300	0.000140	17	236	0
254	27	West Pakistan (CHOP) Canal	122.04	53.65	2.38	0.96	0.300	0.000185	11	302	0
255	28	West Pakistan (CHOP) Canal	138.47	66.14	2.29	0.92	0.300	0.000165	15	395	0
256	29	West Pakistan (CHOP) Canal	138.47	59.44	2.38	0.98	0.300	0.000179	11	150	0
257	30	West Pakistan (CHOP) Canal	143.85	63.40	2.47	0.92	0.300	0.000238	23	197	0
258	31	West Pakistan (CHOP) Canal	139.03	57.91	2.44	0.98	0.311	0.000176	22	198	0
259	32	West Pakistan (CHOP) Canal	226.53	109.42	2.29	0.91	0.311	0.000185	19	388	0
260	33	West Pakistan (CHOP) Canal	399.26	112.78	3.41	1.04	0.311	0.000178	17	1,317	0

Note for bed form:

0 Not observed

1 Plane bed without sed.
movement

2 Ripples

3 Dunes

4 Transitional

5 Plane bed with sediment movement

6 Standing waves

7 Anti dunes

8 Chute - pools