

Use of the Manning Equation for Predicting the Discharge of High-Gradient Canals and Natural Streams

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Abstract. The Manning Equation is used to predict stream or canal discharge from hydraulic radius, slope of the water surface, and a Manning roughness coefficient. Jarrett (1984) proposed that, for high-gradient streams ($S > 0.002$), the Manning roughness coefficient could be predicted from the hydraulic radius and the slope alone. The objective of this study was to develop separate empirical formulae, depending upon climate and stream bank lithology, for predicting the Manning roughness coefficient for high-gradient canals and natural streams from hydraulic radius and slope. The objective was addressed by separating the database used by Jarrett (1984) according to stream bank lithology, and by carrying out new measurements of the Manning roughness coefficient at nine high-gradient stream sites with crystalline (igneous and metamorphic) banks and two high-gradient stream sites with carbonate banks in Haiti, nine high-gradient stream sites with carbonate banks in Utah, and 14 high-gradient canals in Utah. The data were used to develop empirical formulae for predicting the Manning roughness coefficient for (1) continental climate, clastic stream bank (2) tropical climate, crystalline stream bank (3) continental/tropical climate, carbonate stream bank (4) continental climate, earthen canal with grassy bank. The Manning roughness coefficient was a negative function of hydraulic radius for the first case and a positive function for the other cases, suggesting that the increase in turbulent resistance is caused by the roughness of the sediment in the first case, but by the increase in the Reynolds number, which is proportional to the depth, in the other cases.

1. Introduction

In the absence of a gaging station, flume, or weir, the discharge of a canal or natural stream is often predicted using the Manning Equation

$$v = \frac{1.49R^{2/3}S^{1/2}}{n} \quad (1)$$

where v is average velocity (ft/s), R is hydraulic radius (ft), S is the slope of the water surface, and n is the Manning roughness coefficient (Dingman 2009). For streams whose width is much greater than the depth, the hydraulic radius is approximately equal to the average depth. Numerous studies have been devoted to the development of methodology for predicting the Manning roughness coefficient, including visual comparison with

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photographs of streams for which n has been measured (Arcement and Schneider 1989), tables of n for streams of various types and materials (Chow 1959), formulae that take into account various aspects of resistance (Arcement and Schneider 1989), formulae that compute n from sediment grain size and hydraulic radius (Bathurst 1985; Marcus et al. 1992), and statistical formulae that relate n to measurable flow parameters (Dingman and Sharma 1997). Based on studies on mountain streams in Colorado, Jarrett (1984) proposed that, for high-gradient streams ($S > 0.002$), the Manning roughness coefficient could be predicted by

$$n = 0.34S^{0.36}R^{-0.16} \quad (2)$$

(see Fig. 1a). (The formula given in Jarrett (1984) differs slightly because it is based on the energy gradient, rather than the slope of the water surface).

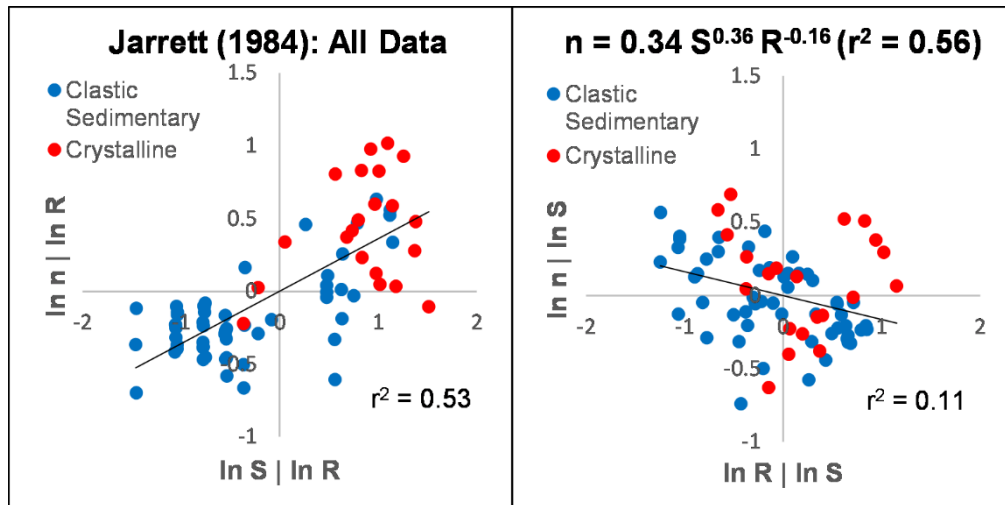


Fig 1a. Jarrett (1984) used 75 discharge measurements at 21 high-gradient stream sites (15 stream sites incising clastic sedimentary bedrock and six sites incising crystalline bedrock) to show that 56% of the variation in the Manning roughness coefficient n could be predicted by the variation in hydraulic radius R and water slope S . On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

The obvious question is: Why does Eq. (2) work at all since it does not take into account sediment size, sediment roundness, stream curvature, or any other aspect of flow resistance? The simple answer is that, in high-gradient streams, a particular combination of slope and hydraulic radius tends to result in the production of sediment of a particular size and roundness. However, the relationship between the inputs (slope and hydraulic radius) and the resulting sediment characteristics must depend upon the type of rock that is being eroded and the climatic regime under which erosion is taking place. The objective of this study was to develop separate empirical formulae, depending upon climate and stream bank lithology, for predicting the Manning roughness coefficient for high-gradient canals and natural streams from hydraulic radius and slope. The objective was addressed by separating the database used by Jarrett (1984) according to stream bank lithology, and by carrying out new measurements of the Manning roughness coefficient at nine high-gradient stream sites with crystalline (igneous and metamorphic) banks and two high-gradient

stream sites with carbonate banks in Haiti, nine high-gradient stream sites with carbonate banks in Utah, and 14 high-gradient canals in Utah (see Tables 1, 2a-b, 3a-b, 4a-b). We are not aware of any previous research on high-gradient canals or on the impact of climate or lithology on the flow resistance of high-gradient streams.

Table 1. Stream sites used by Jarrett (1984)

No.	Stream ¹	Latitude ¹ (°N)	Longitude ¹ (°W)	Stream Bank Lithology ²
1	Arkansas River	38.97722	106.21278	Granite, granodiorite ³
2	Clear Creek	39.76583	105.62556	Felsic gneiss, biotite gneiss, mafic gneiss, schist ³
3	Cottonwood Creek	38.81278	106.22167	Plutonic rock (phaneritic), biotite gneiss, schist ³
4	Crystal River	39.23222	107.22667	Siltstone, shale ⁴
5	Eagle River	39.64944	106.95306	Siltstone, shale ⁴
6	Egeria Creek	40.03667	106.78222	Shale, sandstone ⁴
7	Elk River	40.71750	106.91528	Shale, sandstone ⁴
8	Halfmoon Creek	39.17222	106.38861	Siltstone, sandstone ⁴
9	Hermosa Creek	37.42194	107.84444	Sandstone, conglomerate ⁴
10	Lake Creek	39.06306	107.84444	Basalt, tuff ³
11	Mad Creek	40.56556	106.88861	Sandstone, mudstone ⁴
12	Piedra River	37.22222	107.34222	Sandstone, shale ⁴
13	Rio Grande	37.76694	106.83083	Sandstone, shale ⁴
14	Roaring Fork River	39.54361	107.32889	Siltstone, shale ⁴
15	San Juan River	37.26611	107.01028	Shale, sandstone ⁴
16	Sorth Fork Rio Grande	37.65694	106.64861	Ash-flow tuff ³
17	Trout Creek	40.31222	107.00944	Sandstone, shale ⁴
18	Walton Creek	40.57750	106.78639	Granite, granodiorite ³
19	White River	40.00500	107.82472	Shale, sandstone ⁴
20	Yampa River	40.48361	106.83167	Sandstone, siltstone ⁴
21	Yampa River	40.27972	106.84722	Sandstone, siltstone ⁴

¹Site numbers, names and locations from Jarrett (1984).

²Stream bank lithology obtained by comparing locations with USGS (2005).

³Classified as crystalline.

⁴Classified as clastic sedimentary.

2. Materials and Methods

For the database used by Jarrett (1984), the stream bank lithology was determined by comparing the stream sites with the lithologies given in USGS (2005). If the site was mapped as alluvial material, then the lithology of the adjacent geologic unit (equivalent to the stream bank) was chosen. For all new measurements of the Manning roughness coefficient, the site locations were determined using the Garmin eTrex 10 GPS Receiver, and compared with geological maps for Haiti (Haiti Data 2015) and Utah (Constenius *et al.* 2011).

Stream discharge was measured by dividing streams wider than 10 feet into 20-30 sections of equal width and narrower streams into sections of equal width no smaller than six inches. Both depth and velocity were measured in the middle of each section. The velocity was measured with the USGS Pygmy Current Meter Model 6205 (Rickly Hydrological Company) over a 60-second period at a distance below the water surface equal to 60% of the depth. The slope of the water surface was measured by optical

surveying using the Topcon AT-B2 Auto Level. Eq. (1) was combined with the continuity equation

$$Q = vA \quad (3)$$

where Q is discharge and A is cross-sectional area, and inverted to solve for the Manning roughness coefficient

$$n = \frac{1.49AR^{2/3}S^{1/2}}{Q} \quad (4)$$

Table 2a. Stream sites in Haiti: Site information

No.	Stream	Latitude ¹ (°N)	Longitude ¹ (°W)	Stream Bank Lithology ²
H1	Tenèbre Rivière	19.38272	71.82659	Diorite, tonalite ³
H2	Dlo Carice Rivière	19.38295	71.82793	Diorite, tonalite ³
H3	Maricotte Rivière	19.38890	71.84020	Diorite, tonalite ³
H4	Jean Denante Rivière	19.44689	71.75580	Diorite, tonalite ³
H5	Grande Rivière du Nord	19.63907	72.16946	Andesite, rhyodacite ³
H6	Biquini Rivière	19.63039	72.24890	Andesite, rhyodacite ³
H7	Galois Rivière	19.65517	72.26888	Andesite, rhyodacite ³
H8	Rivière Salé	19.69597	72.29568	Andesite, rhyodacite ³
H9	Limbé Rivière	19.70647	72.39270	Andesite, rhyodacite ³
H10	Namoreau Rivière	19.88595	72.82957	Limestone, marl ⁴
H11	Grande Rivière	19.88653	72.83005	Limestone, marl ⁴

¹Coordinate system is WGS 84.

²Stream bank lithology obtained by comparing locations with Haiti Data (2015).

³Classified as crystalline.

⁴Classified as carbonate.

Table 2b. Stream sites in Haiti: Stream properties

No.	Water Slope, S	Hydraulic Radius, R (ft)	Manning Roughness Coefficient, n
H1	0.0048	0.821	0.406
H2	0.0040	0.280	0.099
H3	0.0032	0.704	0.562
H4	0.0056	0.441	0.163
H5	0.0023	0.457	0.066
H6	0.0114	0.124	0.108
H7	0.0036	0.592	0.115
H8	0.0023	0.178	0.036
H9	0.0046	0.556	0.049
H10	0.0364	0.430	0.154
H11	0.0071	0.568	0.058

3. Results

The stream sites used by Jarrett (1984) were initially separated into 15 sites incising clastic sedimentary bedrock and six sites incising crystalline bedrock. The tendency of the crystalline stream sites to plot in the region of positive residuals of $\ln n$ and the preponderance of points above the best-fit partial regression lines indicate that both $\ln S$

and $\ln R$ underpredict $\ln n$ for these sites (see Fig. 1a). This result is consistent with the tendency of crystalline rocks to erode into larger particles, compared with clastic sedimentary rocks, and suggests that different multiple regression formulae should be used for the two generalized lithologies. However, removing the sites with crystalline stream banks only slightly improved the fit of the multiple regression formula (from $r^2 = 0.56$ to $r^2 = 0.58$, see Figs. 1a-b), probably because most of the measurements (54 out of 75) were on sites with clastic stream banks. When only the sites with clastic sedimentary stream banks were considered, there was some tendency for the sites with coarser-grained (primarily sandstone) banks to plot in the region of positive residuals of $\ln n$ and for the sites with finer-grained (primarily shale/siltstone) banks to plot in the region of negative residuals of $\ln n$ (see Fig. 1b), which is consistent with the greater flow resistance that should result from larger sediment. Separating the sites with sandstone and shale/siltstone banks resulted in an improved fit for sandstone sites alone ($r^2 = 0.63$), but not for shale/siltstone sites alone ($r^2 = 0.53$). Finally, considering only the stream sites with crystalline banks resulted in a poor regression fit ($r^2 = 0.10$, see Fig. 1c), probably due to the very wide variety of crystalline rock types represented in the database (see Table 1).

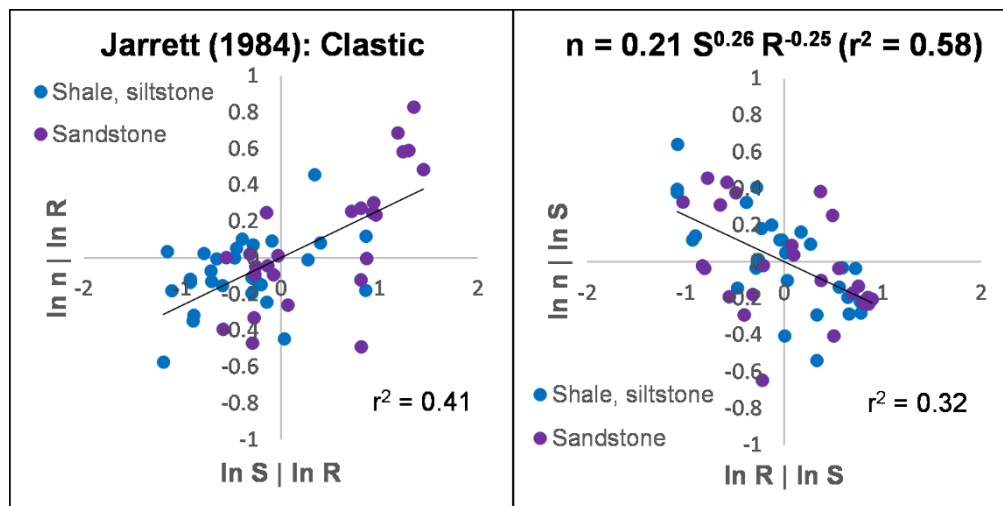


Fig 1b. Based on the data set of Jarrett (1984), the ability of hydraulic radius R and water slope S to predict the Manning roughness coefficient n for high-gradient streams is slightly improved by considering only those stream sites that are incising clastic sedimentary rock. Separating the sites with sandstone and shale/siltstone banks resulted in an improved fit for sandstone sites alone ($r^2 = 0.63$), but not for shale/siltstone banks alone ($r^2 = 0.53$). On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

When the 21 measurements carried out by Jarrett (1984) at high-gradient stream sites incising crystalline bedrock were combined with the measurements at nine high-gradient sites incising crystalline bedrock in Haiti, the multiple regression fit was very poor ($r^2 = 0.01$, see Fig. 2a), probably due to mixing stream sites from continental and tropical weathering environments. Restricting the stream sites to Haiti significantly improved the ability of slope and hydraulic radius to predict the Manning roughness coefficient ($r^2 = 0.46$, see Fig. 2b). The regression formula tends to underpredict the Manning roughness

coefficient for streams incising diorite/tonalite and overpredict the Manning roughness coefficient for streams incising andesite/rhyodacite, which is consistent with the larger sediment derived from coarser-grained rocks (diorite/tonalite).

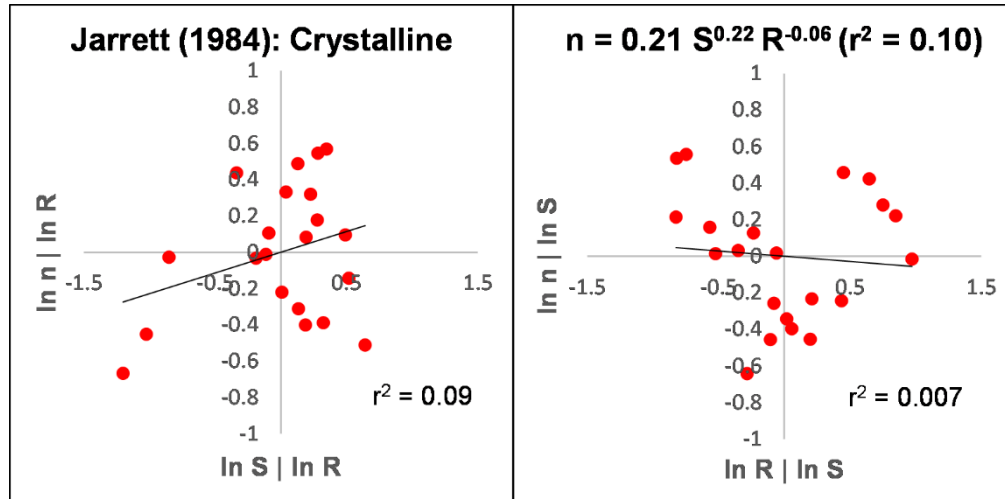


Fig 1c. Based on the data set of Jarrett (1984), the ability of hydraulic radius R and water slope S to predict the Manning roughness coefficient n for high-gradient streams is greatly reduced by considering only those stream sites that are incising crystalline rock. The poor fit is probably due to the wide range of crystalline rock types at the Colorado sites (see Table 1). On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

Table 3a. Stream sites in Utah: Site information

No.	Stream	Latitude ¹ (°N)	Longitude ¹ (°W)	Stream Bank Lithology ²
U1	Right Fork Hobble Creek	40.16313	111.49931	Limestone
U2	Right Fork Hobble Creek	40.16721	111.49117	Limestone
U3	Tributary, South Fork Provo River	40.33374	111.52430	Limestone
U4	South Fork Provo River	40.34766	111.54687	Limestone
U5	Tributary, North Fork Provo River	40.37314	111.58029	Limestone
U6	North Fork Provo River	40.38349	111.57239	Limestone
U7	North Fork Provo River	40.37843	111.56836	Limestone
U8	Tributary, Hobble Creek	40.13144	111.52839	Limestone
U9	Tributary, Hobble Creek	40.13271	111.52518	Limestone

¹Coordinate system is WGS 84.

²Bedrock lithology obtained by comparing locations with Constenius et al. (2011).

Combining 13 discharge measurements carried out at high-gradient stream sites incising carbonate bedrock in Utah (see Table 3a) with two measurements at high-gradient sites incising carbonate bedrock in Haiti (see Table 2a) showed that 47% of the variation in the Manning roughness coefficient could be predicted by the variation in hydraulic radius and slope (see Fig. 3). The tendency of the stream sites in Haiti to plot in the region of negative residuals of $\ln n$ and below the best-fit regression line indicates that both $\ln S$ and $\ln R$ overpredict n for these sites. This result is consistent with the tendency of carbonate rocks to erode into smaller and more rounded particles in the tropical climate of Haiti.

Table 3b. Stream sites in Utah: Stream properties

No.	Water Slope, S	Hydraulic Radius, R (ft)	Manning Roughness Coefficient, n
U1	0.0114	0.542	0.140
U1	0.0114	0.940	0.310
U2	0.0175	0.670	0.116
U2	0.0175	1.581	0.534
U2	0.0175	0.466	0.070
U3	0.0491	0.281	0.195
U4	0.0116	0.396	0.066
U5	0.0387	0.525	0.137
U5	0.0387	0.552	0.086
U6	0.0552	0.491	0.108
U7	0.0564	0.883	0.685
U8	0.1265	0.364	0.670
U9	0.0584	0.226	0.554

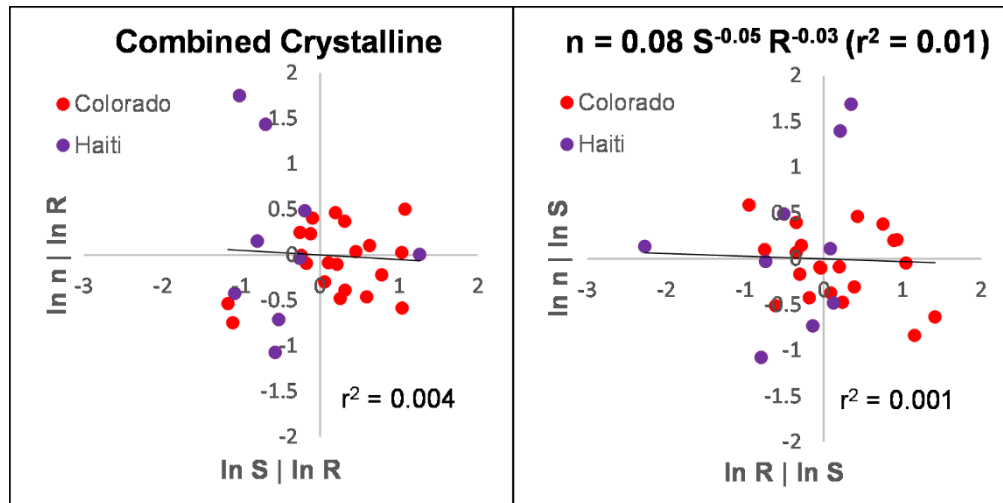


Fig 2a. Combining the 21 discharge measurements carried out by Jarrett (1984) at six high-gradient stream sites incising crystalline bedrock in Colorado with nine measurements at nine high-gradient sites incising crystalline bedrock in Haiti showed that only 1% of the variation in the Manning roughness coefficient n could be predicted by the variation in hydraulic radius R and water slope S . The poor fit is probably due to the wide range of crystalline rock types and the different weathering environments. On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

The 14 high-gradient canal sites in Utah included four types of canals (see Fig. 4, Table 4a). For the four canals lined with concrete with a trowel finish, all of the measured values of the Manning roughness coefficient (0.027-0.066) exceeded the typical maximum value of $n = 0.015$ (Chow, 1959, see Tables 4a-b). The higher values for high-gradient canals probably resulted from the tendency of rapid flows to strip away the trowel finish to expose the rough aggregate underneath. In the case of the Mt. Nebo Canal (see Table 4a), the very rough sides in the lower part of the canal wall could result from shallow, supercritical flows or simply from the more frequent exposure to flowing water (see left-

hand side of Fig. 4). For the five earthen canals with cobble bottoms and banks, it was also found that all of the measured values of the Manning roughness coefficient (0.083-0.104) exceeded the typical maximum value of $n = 0.050$ (Chow, 1959, see Tables 4a-b). For these canals, the cobbles were not transported by flow, but were simply placed for decoration and stabilization. For steep canals, it is likely that, in order to stabilize the canal banks, larger cobbles would be used than would be typical in much more common low-gradient canals, leading to a larger roughness coefficient. For the five earthen canals with grassy, weedy banks, the measured values of the Manning roughness coefficient (0.027-0.049) were not much different from the typical range (0.025-0.040). For these canals, hydraulic radius and slope were excellent predictors of the roughness coefficient ($r^2 = 0.88$, see Fig. 5), with hydraulic radius being a much stronger predictor (see right-hand side of Fig. 5). The single canal with fewer weeds did not appear to be an outlier (see Fig. 5).

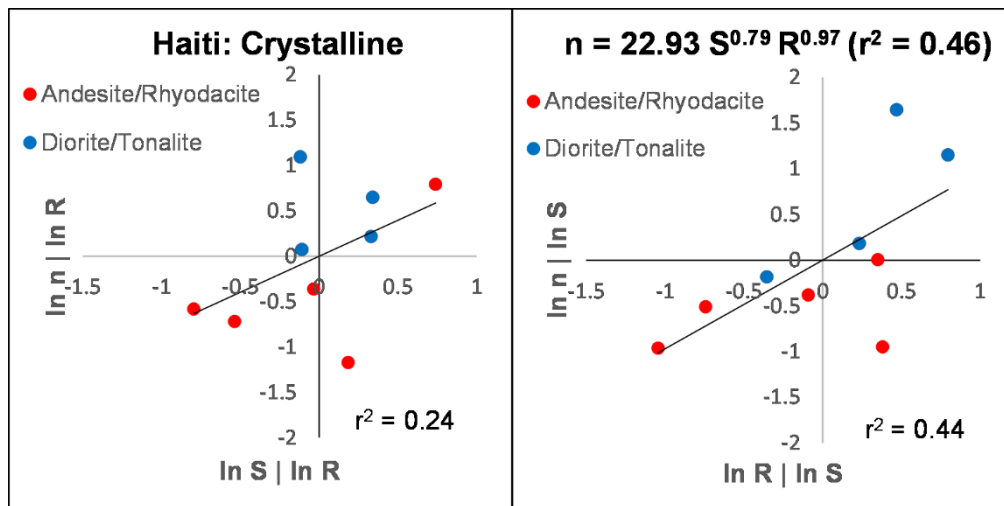


Fig. 2b. The ability of hydraulic radius R and water slope S to predict the Manning roughness coefficient n for high-gradient streams incising crystalline bedrock is greatly improved by considering only stream sites in Haiti and excluding stream sites in Colorado. On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

4. Discussion

The chief difference among the multiple regression formulae is that, in that, in the case of streams with clastic stream banks, the Manning roughness coefficient is a negative function of the hydraulic radius, while, in all other cases, the roughness coefficient is a positive function of the hydraulic radius (see Table 5a). In the case of clastic sediment, which tends to be more angular, turbulent resistance is mainly caused by the roughness of the sediment, the effect of which decreases as the water becomes deeper. However, in the case of crystalline or carbonate sediment, which tends to be more rounded, turbulent resistance is mainly caused by the increase in the Reynolds number, which is proportional to the depth. By combining the formulae for the roughness coefficient with Eq. (1), it can be seen that average velocity is an positive function of hydraulic radius for streams with clastic banks and earthen canals with grassy banks, but a negative function of hydraulic radius for streams with crystalline or carbonate banks (see Table 5b).

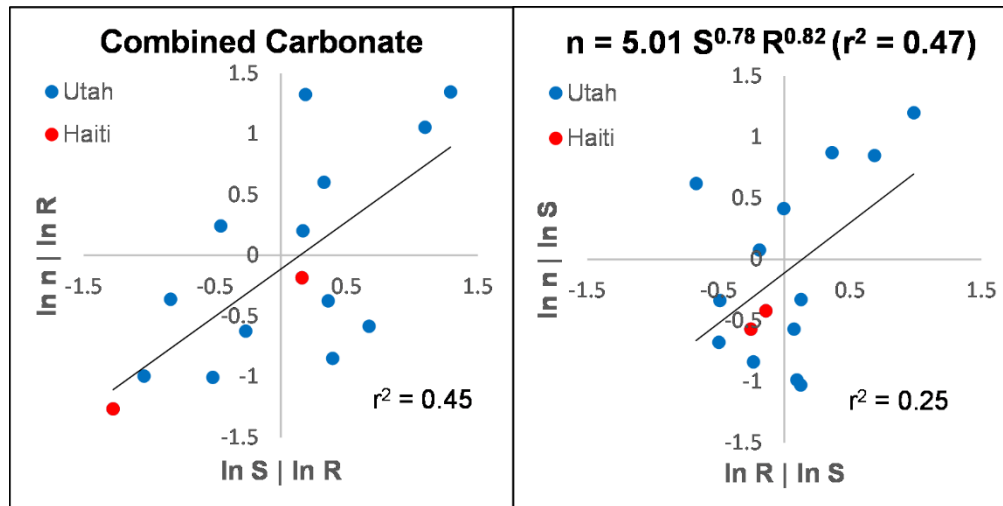


Fig 3. Combining 13 discharge measurements carried out at high-gradient stream sites incising carbonate bedrock in Utah with two measurements at high-gradient sites incising carbonate bedrock in Haiti showed that 47% of the variation in the Manning roughness coefficient n could be predicted by the variation in hydraulic radius R and water slope S . On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

Table 4a. Canal sites in Utah: Site information

No.	Canal	Latitude ¹ (°N)	Longitude ¹ (°W)
Concrete, trowel finish ²			
UC1	Murdock Canal	40.31346	111.65699
UC2	Mt. Nebo Canal	39.79838	111.81807
UC3	Mt. Nebo Canal	39.79838	111.81807
UC4	Mt. Nebo Canal	39.79838	111.81807
Earth, winding, cobble bottom, clean (non-weedy) sides ²			
UC5	BYU Canal	40.25209	111.64698
UC6	UVU South Canal	40.27601	111.71126
UC7	UVU South Canal	40.27601	111.71126
UC8	UVU South Canal	40.27601	111.71126
UC9	UVU North Canal	40.27676	111.71182
Earth, winding, grass, some weeds ²			
UC10	Larry H. Miller Ditch	40.28418	111.69053
Earth, winding, grass, dense weeds ²			
UC11	Lions Park Canal	40.25276	111.67612
UC12	Lions Park Canal	40.25236	111.67611
UC13	Lions Park Canal	40.25147	111.67604
UC14	Lions Park Canal	40.25058	111.67589

¹Coordinate system is WGS 84.

²Descriptions follow the categories in Chow (1959).

Table 4b. Canal sites in Utah: Flow properties

No.	Water Slope, S	Hydraulic Radius, R (ft)	Manning Roughness Coefficient, n
Min $n = 0.011$, Normal $n = 0.013$, Max $n = 0.015$ ¹			
UC1	0.0019	0.086	0.027
UC2	0.3669	0.246	0.050
UC3	0.1755	0.288	0.046
UC4	0.0416	0.296	0.066
Min $n = 0.030$, Normal $n = 0.040$, Max $n = 0.050$ ¹			
UC5	0.0019	0.167	0.104
UC6	0.0240	0.029	0.095
UC7	0.0240	0.029	0.115
UC8	0.0240	0.029	0.095
UC9	0.0090	0.149	0.083
Min $n = 0.025$, Normal $n = 0.030$, Max $n = 0.033$ ¹			
UC10	0.0120	0.506	0.034
Min $n = 0.030$, Normal $n = 0.035$, Max $n = 0.040$ ¹			
UC11	0.0052	0.726	0.049
UC12	0.0060	0.463	0.033
UC13	0.0043	0.506	0.029
UC14	0.0059	0.490	0.027

¹Typical minimum, normal and maximum values of Manning roughness coefficient for a given type of canal are taken from Chow (1959). Note that the right-hand column is the measured Manning roughness coefficient at a particular canal site.



Fig. 4. Many canals in Utah were constructed along very steep slopes. In the left-hand photo, the Mt. Nebo Canal (Site UC2, see Table 4a) descends along a slope $S = 0.3669$. In the lower part of the canal wall, the concrete finish has been stripped away to expose the rough aggregate underneath. In the right-hand photo, the Lions Park Canal (Site UC11, see Table 4a) is an earthen canal with banks of grass and dense weeds, descending at a slope $S = 0.0052$.

The provisional formulae for the Manning roughness coefficient assume the most generalized categories that are reasonably possible based upon the results of this study (see Table 5a-b). For example, measurements in Colorado and Utah are assumed to be representative of a continental climate, while measurements in Haiti are assumed to be representative of a tropical climate. Moreover, diorite/tonalite and andesite/rhyodacite are assumed to be representative of crystalline rock (see Tables 2a, 5a-b), while limestone and marl are assumed to be representative carbonate rocks (see Tables 2a, 3a, 5a-b). Clearly, more studies are needed that would either refine the categories or confirm the generalized categories. Up to this point, studies of flow resistance in mountain streams have neither documented stream bank lithology nor used lithology as an experimental control (e.g., Jarrett 1984; Reid and Hickin 2008; Asano et al. 2012; Zink and Jennings 2014).

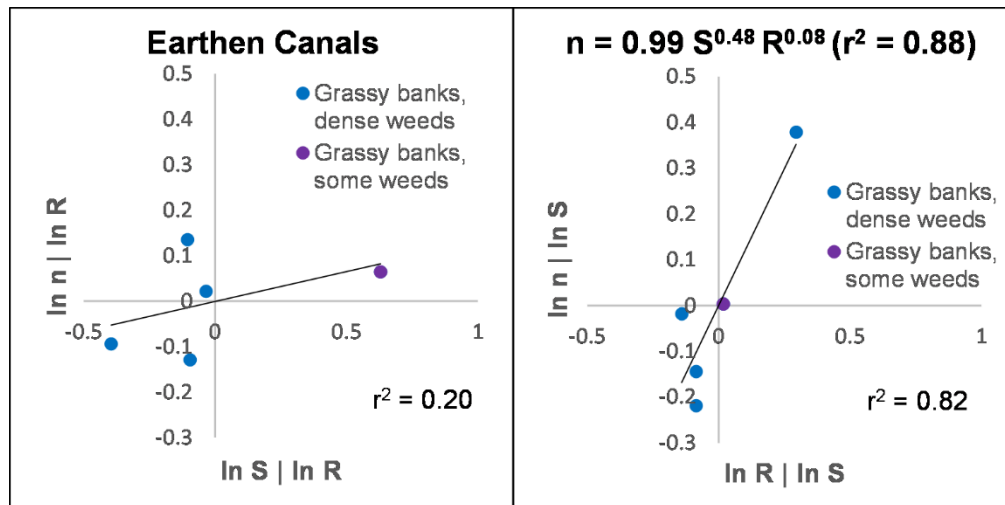


Fig 5. Based on discharge measurements carried out at five high-gradient earthen canal sites in Utah, 88% of the variation in the Manning roughness coefficient n could be predicted by the variation in hydraulic radius R and water slope S . All earthen canal sites had grassy banks with some or dense weeds. On the left-hand plot, the residual of regressing $\ln n$ against $\ln R$ is shown as a function of the residual of regressing $\ln S$ against $\ln R$. On the right-hand plot, the residual of regressing $\ln n$ against $\ln S$ is shown as a function of the residual of regressing $\ln R$ against $\ln S$.

Table 5a. Provisional Formulae for Manning Roughness Coefficient in High-Gradient Canals and Natural Streams

Climate	Substrate	Formula ^{1,2}
Natural Streams		
Continental	Clastic Rock	$n = e^{(-1.58 \pm 0.22)} S^{(0.26 \pm 0.04)} R^{(-0.25 \pm 0.05)}$
Tropical	Crystalline Rock	$n = e^{(3.13 \pm 3.36)} S^{(0.79 \pm 0.58)} R^{(0.97 \pm 0.45)}$
Continental/Tropical	Carbonate Rock	$n = e^{(1.61 \pm 1.04)} S^{(0.78 \pm 0.25)} R^{(0.82 \pm 0.41)}$
Canals		
Continental	Earth, Grassy Banks	$n = e^{(-0.01 \pm 0.01)} S^{(0.48 \pm 0.73)} R^{(0.08 \pm 0.02)}$

¹ n = Manning roughness coefficient, S = slope of water surface, R = hydraulic radius (ft)

² Uncertainties are standard errors.

It is important to reinforce that the Manning roughness coefficient for mountain streams can often vastly exceed the commonly quoted range of $n = 0.030$ - 0.070 (Dingman

2009). The largest value found in this study was $n = 0.685$ (see Table 3b). Jarrett (1984) measured the roughness coefficient as high as $n = 0.159$, while Reid and Hickin (2008) measured a roughness coefficient of $n = 7.95$. Underestimating the Manning roughness coefficient can result in an overestimation of the resources available in mountain streams, such as the fluvial power available for generation of electricity. Moreover, underestimating the Manning roughness coefficient overestimates the ability of a mountain stream to accommodate an increase in stream discharge, thus underestimating the flooding potential of mountain streams under the impact of severe storms.

Table 5b. Manning Equations based upon Provisional Formulae for Manning Roughness Coefficient in High-Gradient Canals and Natural Streams

Climate	Substrate	Manning Equations ^{1,2}
Natural Streams		
Continental	Clastic Rock	$v = 7.24R^{0.92}S^{0.24}$
Tropical	Crystalline Rock	$v = 0.065R^{-0.30}S^{-0.29}$
Continental/Tropical	Carbonate Rock	$v = 0.30R^{-0.15}S^{-0.28}$
Canals		
Continental	Earth, Grassy Banks	$v = 1.51R^{0.59}S^{0.02}$

¹The Manning Equations were obtained by combining Eq. (1) with Table 5a.

² v = average velocity (ft/s), R = hydraulic radius (ft), S = slope of water surface

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